This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page.

This file or an individual page shall not be considered a certified document.

REFERENCE

CONTENTS

DESCRIPTION

LEGEND (SOIL & ROCK)

ROCK TEST RESULTS CORE PHOTOGRAPH(S)

SITE PHOTOGRAPH(S)

SUPPLEMENTAL LEGEND (GSI)

BORE LOG(S) & CORE REPORT(S)

TITLE SHEET

SITE PLAN

PROFILE(S) CROSS SECTION(S)

SHEET NO.

2Α

4-6

7-9 10-17

18-19

20-22

23

401

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

2KU 1F () 1		DFDI	ACE DI	RIDGE 46	OVED
	VER ON			dDGL 40	OVER
SITE DESCI	RIPTION				

STATE PROJECT REFERENCE NO. 24 B-4962

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (1991) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOL. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES BY ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

B. SMITH, PG

B. WORLEY, PG

M. SHIPMAN, EI L. GONZALEZ

D. SUTTON

INVESTIGATED BY <u>B. SMITH, PG</u>

DRAWN BY _B. SMITH, PG

CHECKED BY B. WORLEY, PG

SUBMITTED BY __B. SMITH, PG

DATE __*JULY*, 2018

Prepared in the Office of:



NC FIRM LICENSE No: P-0339 and C-487 504 Meadowlands Drive Hillsborough, NC 27278 (919) 732-3883 (919) 732-6676 (FAX)



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DI586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. <u>GAP-GRADED</u> - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN IGNEOUS AND METAMORPHIC POCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.	POCK (CB). WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE,	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	NON-CRYSTALLINE ONE 15S. GABBRO, SCHIST, ETC. FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000 d00000 00000 00000 00000 00000 00000 0000	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING SILT-	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED (CP) SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*10 59 MX GRANULAR CLAY PEAT GRANULAR CLAY PEAT SOILS	GRANULAR SILT - CLAY	WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
25 MX 25 MX 25 MX 35 MX 35 MX 35 MX 36 M 3	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN.	HORIZONTAL.
LL 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 501L5 WITH	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
P! 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN MODERATE OPERATE	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX W W 4 MX 8 MX 12 MX 16 MX NU MX AMUUN 15 UF SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL AND FINE SILTY OR CLAYEY SILTY CLAYEY MATTER	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SAND GRAVEL AND SAND SOILS SOILS	▼ STATIC WATER LEVEL AFTER <u>24</u> HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM
GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE	√Pw PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA FINANCIAL PROPERTY OF THE PRO	(MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	PARENT MATERIAL.
AS SUBGRADE PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ;PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
DANCE OF CTANDARD DANCE OF UNICONETNED	THIS CECENICOUS STILLOUS	(MOD.SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (IN-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
VERY LOOSE 4.4	SPT C SLOPE INDICATOR	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY LOOSE 4 TO 10	SOIL SYMBOL OPT DAT TEST BORING INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS
MATERIAL MEDIUM DENSE 10 10 30 N/A	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	A TIME TO ADMINI EMBRICATION OF TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u>	OF AN INTERVENING IMPERVIOUS STRATUM.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2	A ALLINIA SOIL BOUNDARY A PIEZOMETER	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE
HARD > 30 > 4	INSTALLATION — SPI NOTHLUE	ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	LICED IN THE TOP 2 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE GUIDE FOR FIELD MOISTURE DESCRIPTION (ATTERBERG LIMITS) DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
CATURATED HOUSE A FOUND HERY HET HOUSE IN	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC PLASTIC PROMITE AND A	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE, CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE / - WET - (W) SEMISULID; REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL W - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BL-4
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS	(N: 846524 E: 1978896)
- MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION: 516.8 FEET
OM OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	FIAD = FILLED IMMEDIATELY AFTER DRILLING
- DRT - (U) ATTAIN OPTIMUM MOISTURE	CME-55 CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	
PLASTICITY	8* HOLLOW AUGERS	INDURATION	
PLASTICITY INDEX (PI) DRY STRENGTH	X CME-550X HARD FACED FINGER BITS X-N Q2	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM	X CASING X W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST		
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-1-
	<u> </u>	<u>.</u>	!

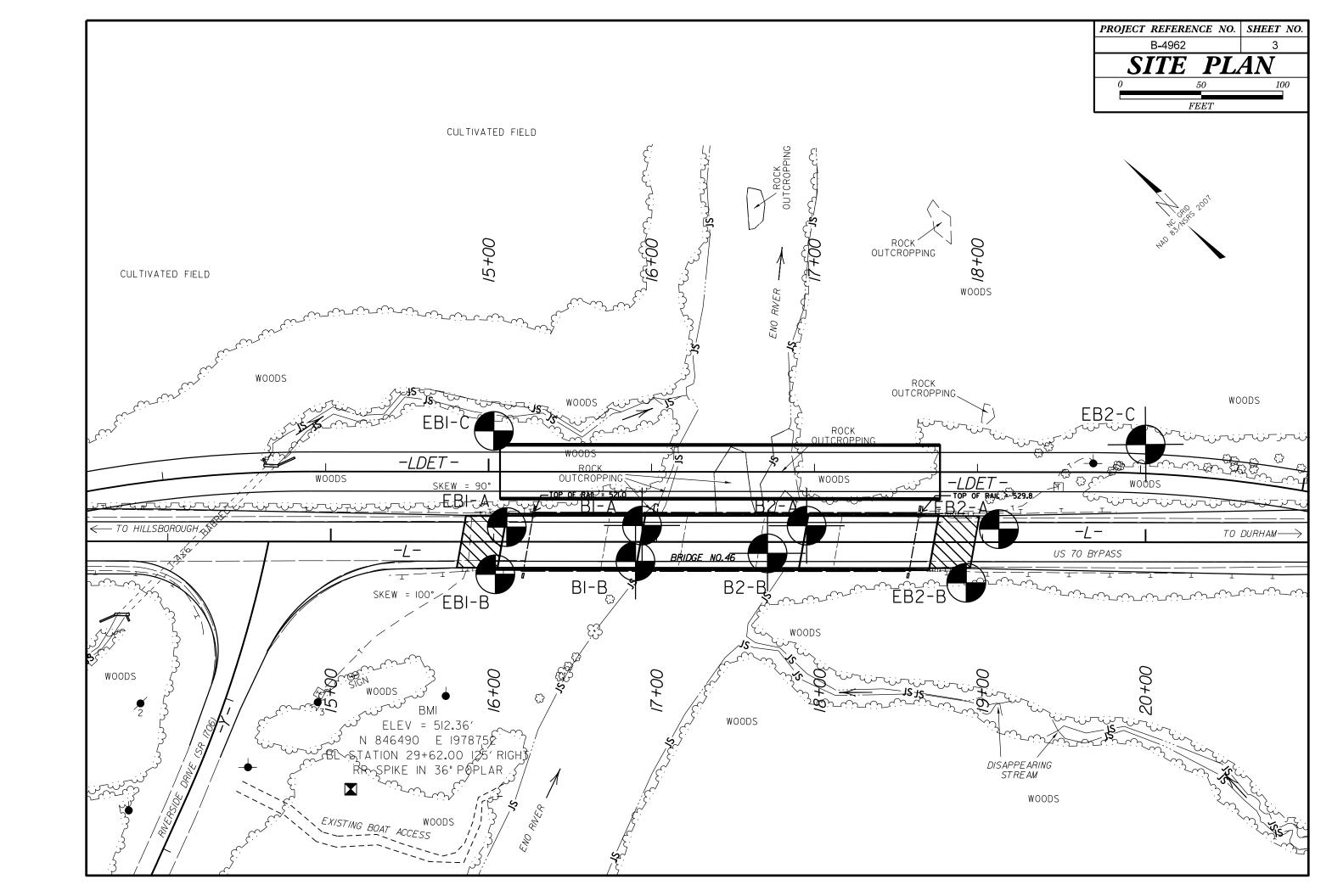
JECT REFERENCE NO.	SHEET NO.
4962	2A

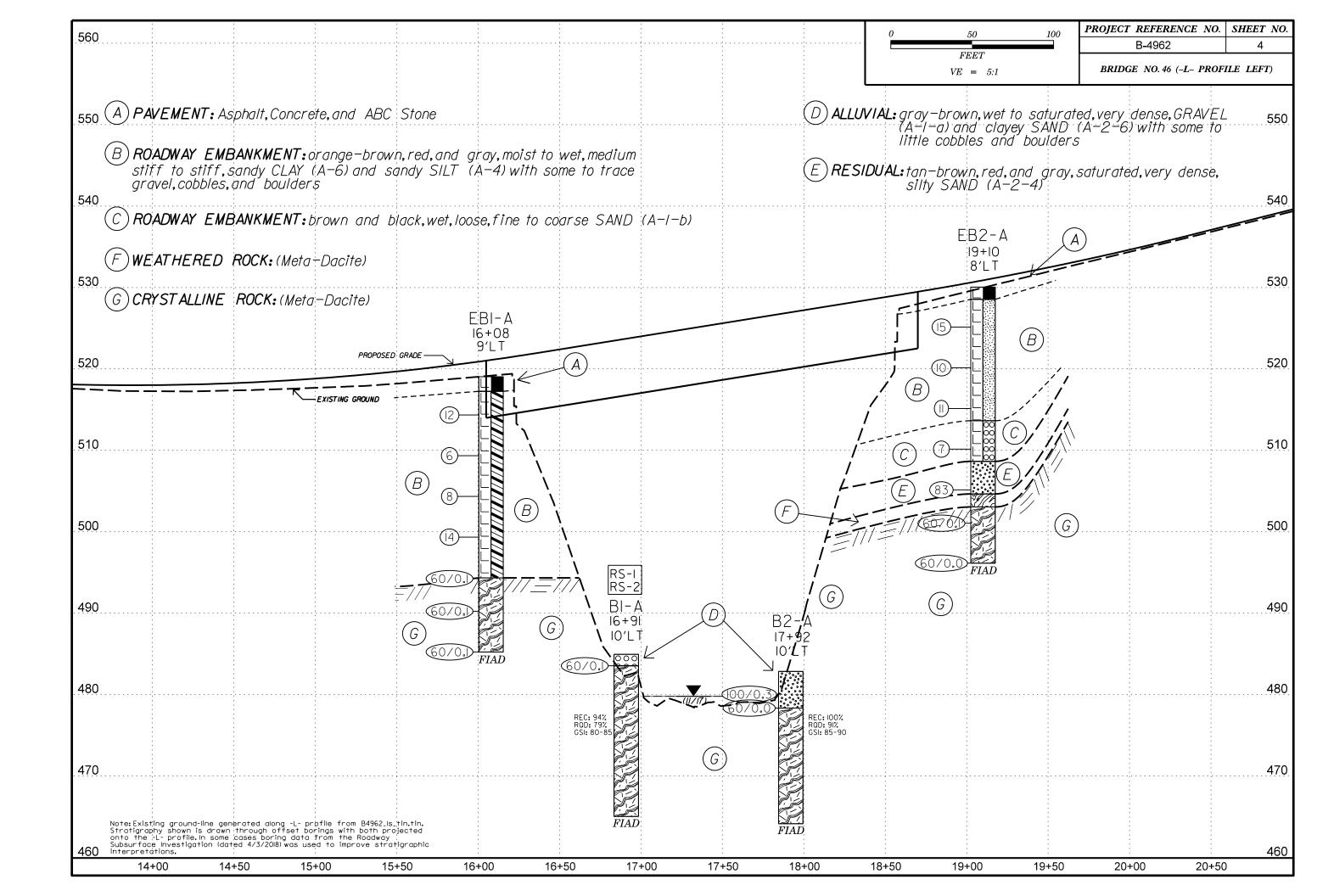
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

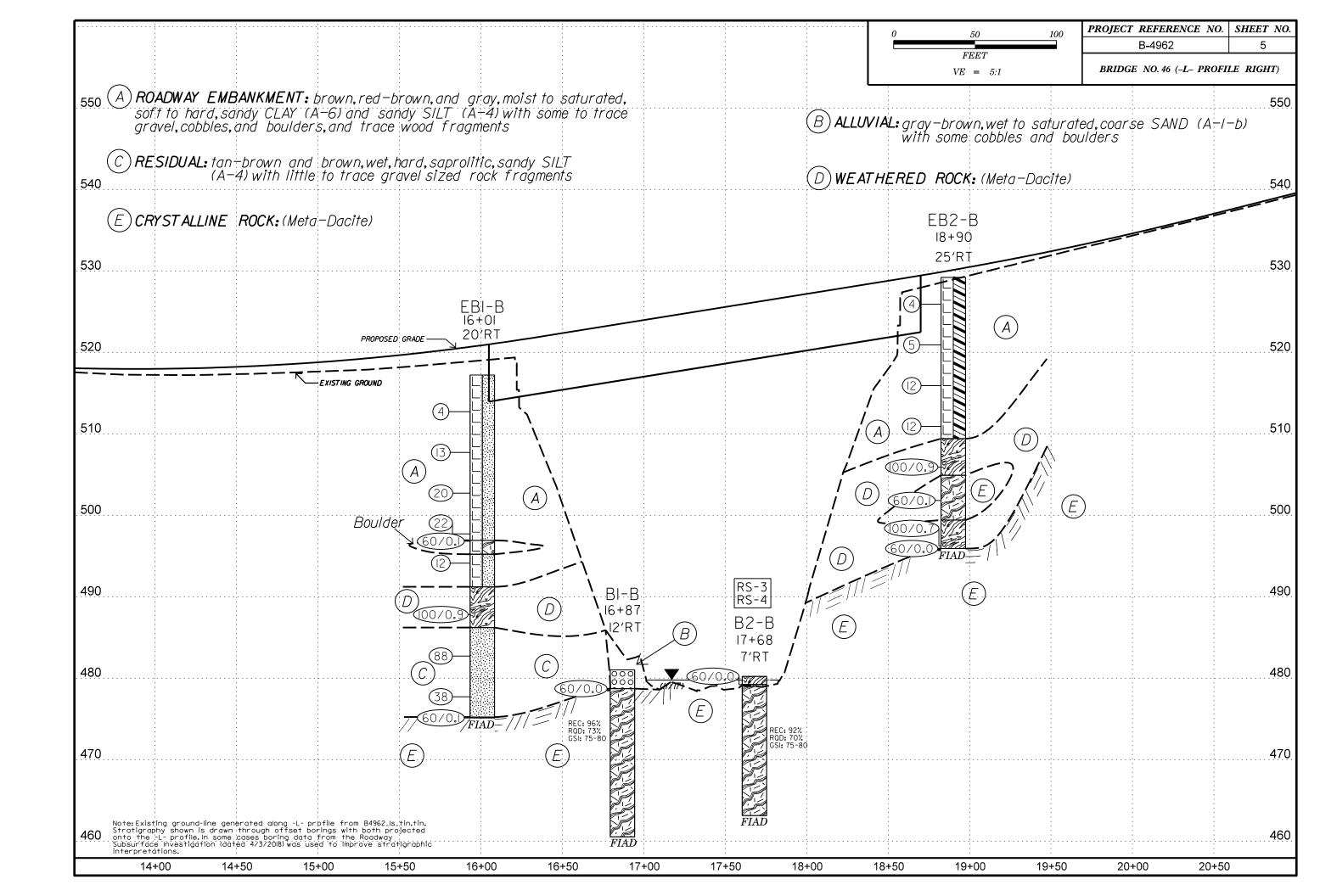
SUBSURFACE INVESTIGATION

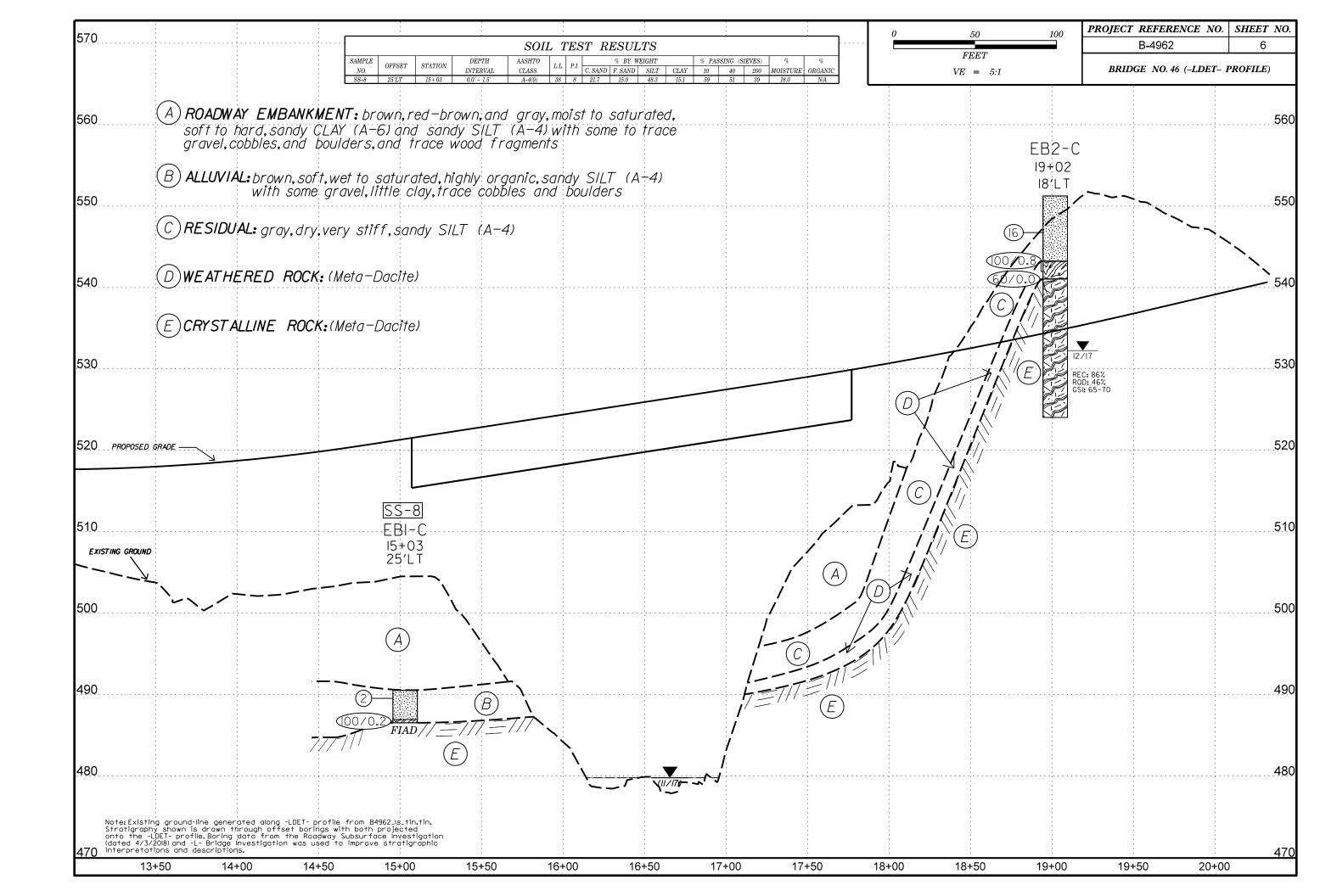
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

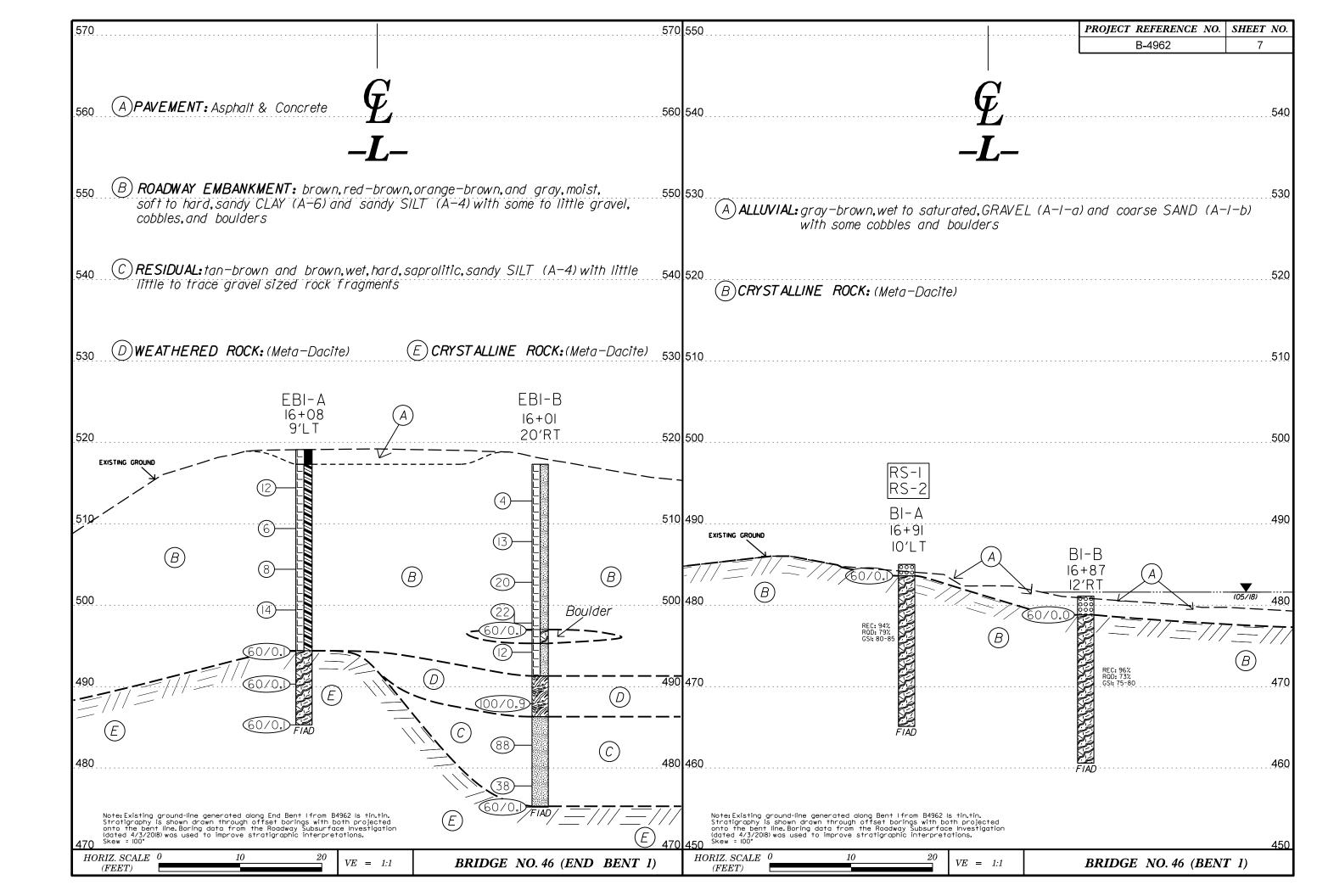
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	nted Ro	ock Mass (Marinos and Hoek, 2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		s p		ν Φ Ο	S O O	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS	VERY GOOD Very rough, fresh unweathered surfaces GOOD Rough, slightly weathered, iron stained surfaces	FAIR Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfa- with compact coatings or fillings or angular fragments	VERY POOR Slickensided, highly weathered surf with soft clay coatings or fillings	Execution of the lithology, structure and sortage conditions (barticularly of the pedding planes), choose a pox in the chart. Tocate the bosition in the pox that corresponds to the condition of the discontinuities and estimate the average value of GSI from the controlled failures. Mooth, moderately these mild opening of the strength of some controlled failures. Where authorough of surfaces with angular of controlled soft colled soft soft colled soft soft soft soft soft soft soft soft
STRUCTURE		DECREASING SU	URFACE QU	JALITY ===	>	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities BLOCKY - well interlocked un-	PIECES 	90 80		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability. 60
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks	OCKING OF ROCK	70 60	50			8. Sand- stone with thin inter- layers of siltstone siltstone amounts C. Sand- stone and stone and siltstone or silty shale with sand- stone layers stone layers stone with siltstone or clayey shale with sandstone layers 40
formed by 4 or more joint sets BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	 ASING INTERLOC 		40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H. F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	 			20		G. Undisturbed silty or clayey shale with or clayey shale forming a chaotic structure with pockets of clay. Thin layers of spatial protectioned sintly protections of spatial protections and protections are protected.
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	٧	N/A N/A		$\langle \ / \ \rangle$	10	Into small rock pieces. → Means deformation after tectonic disturbance DATE: 8-19-1

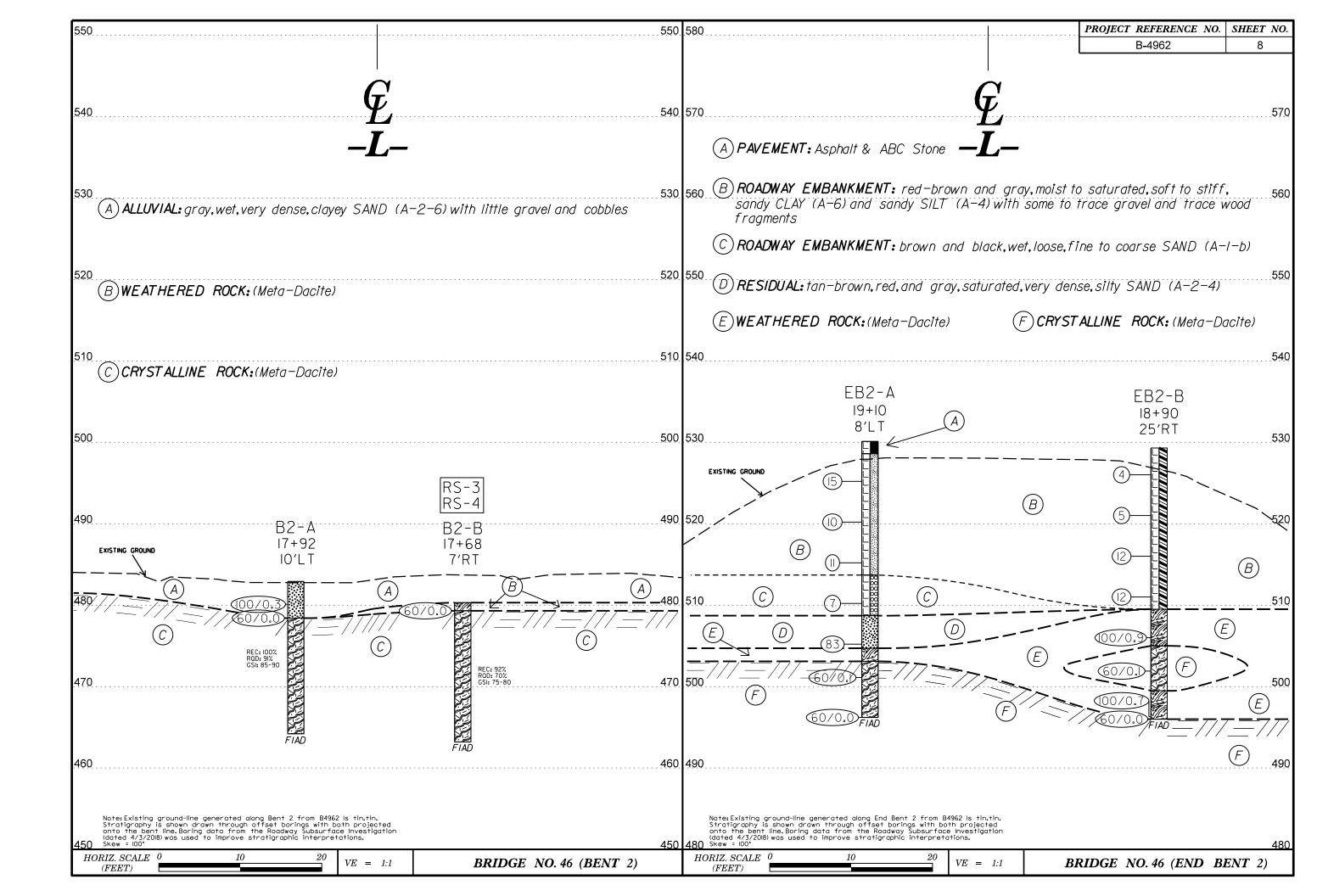


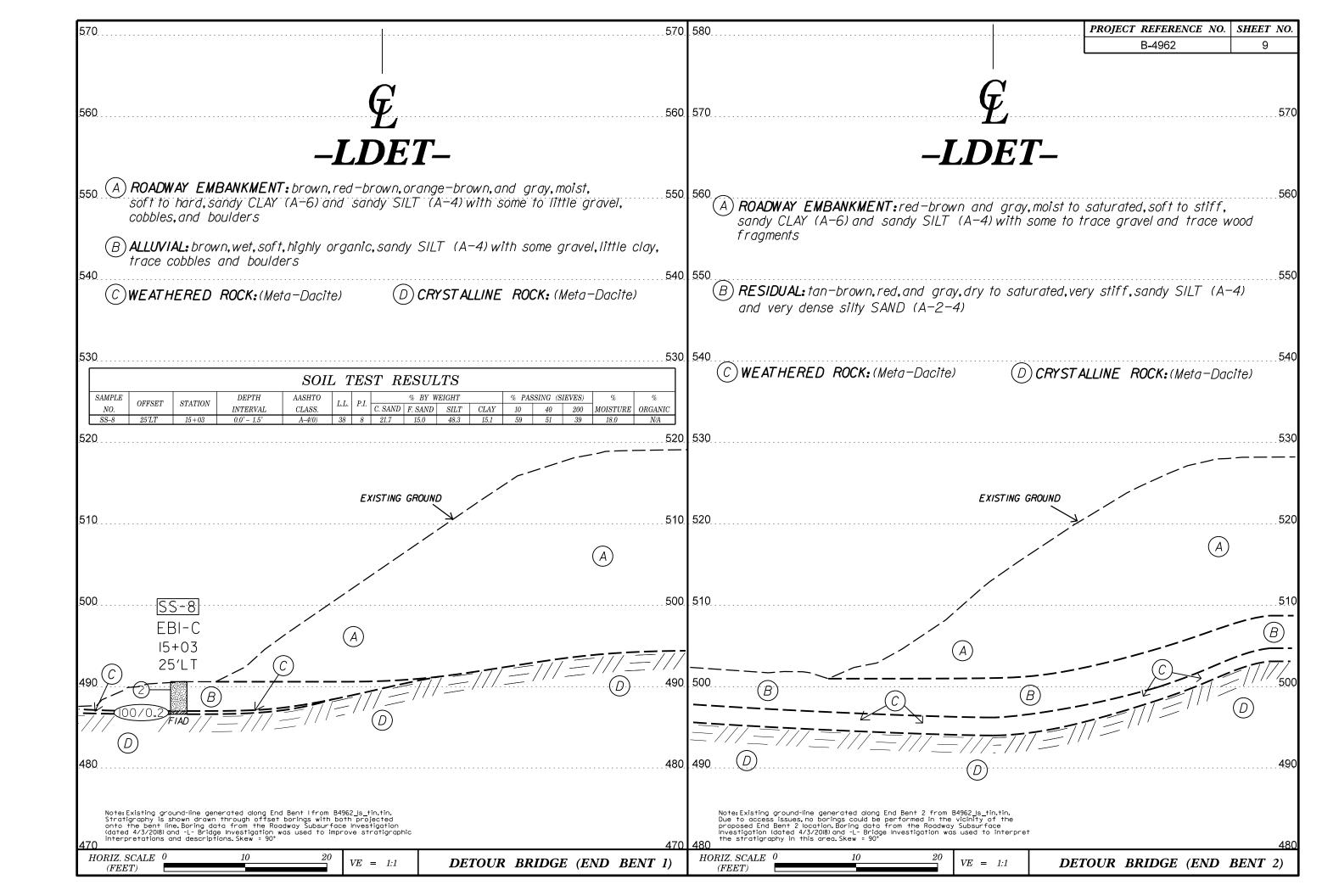


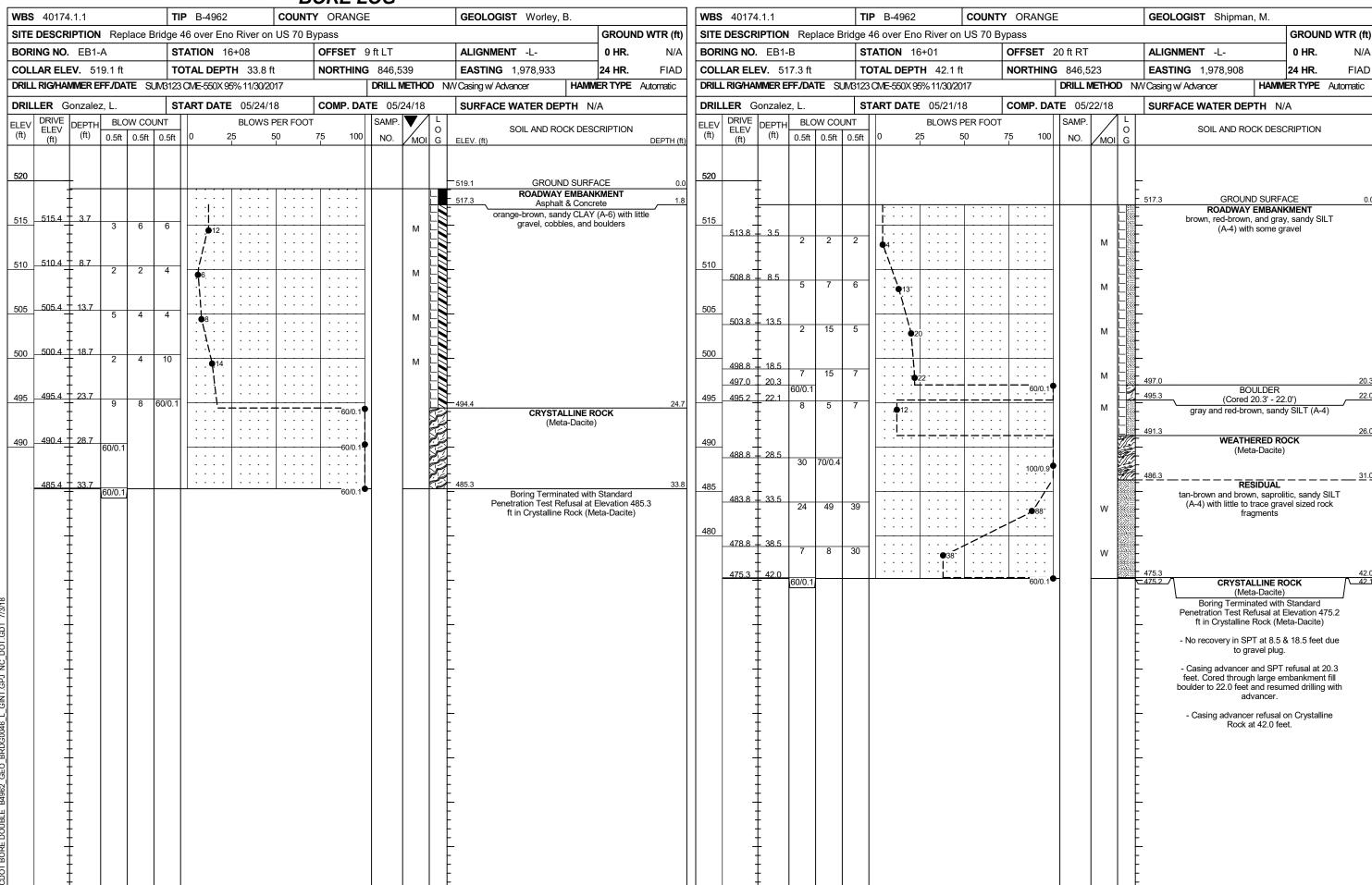


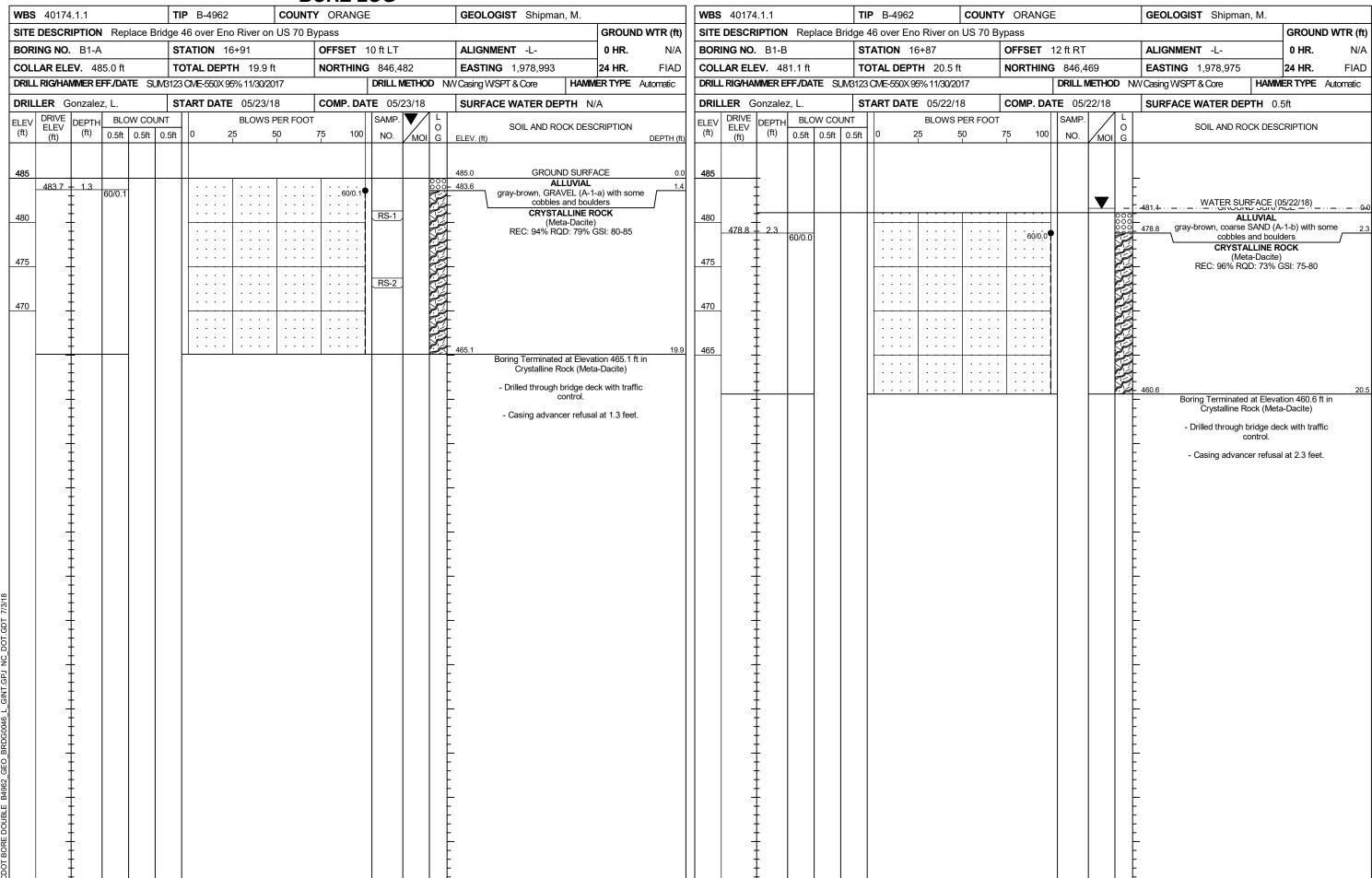












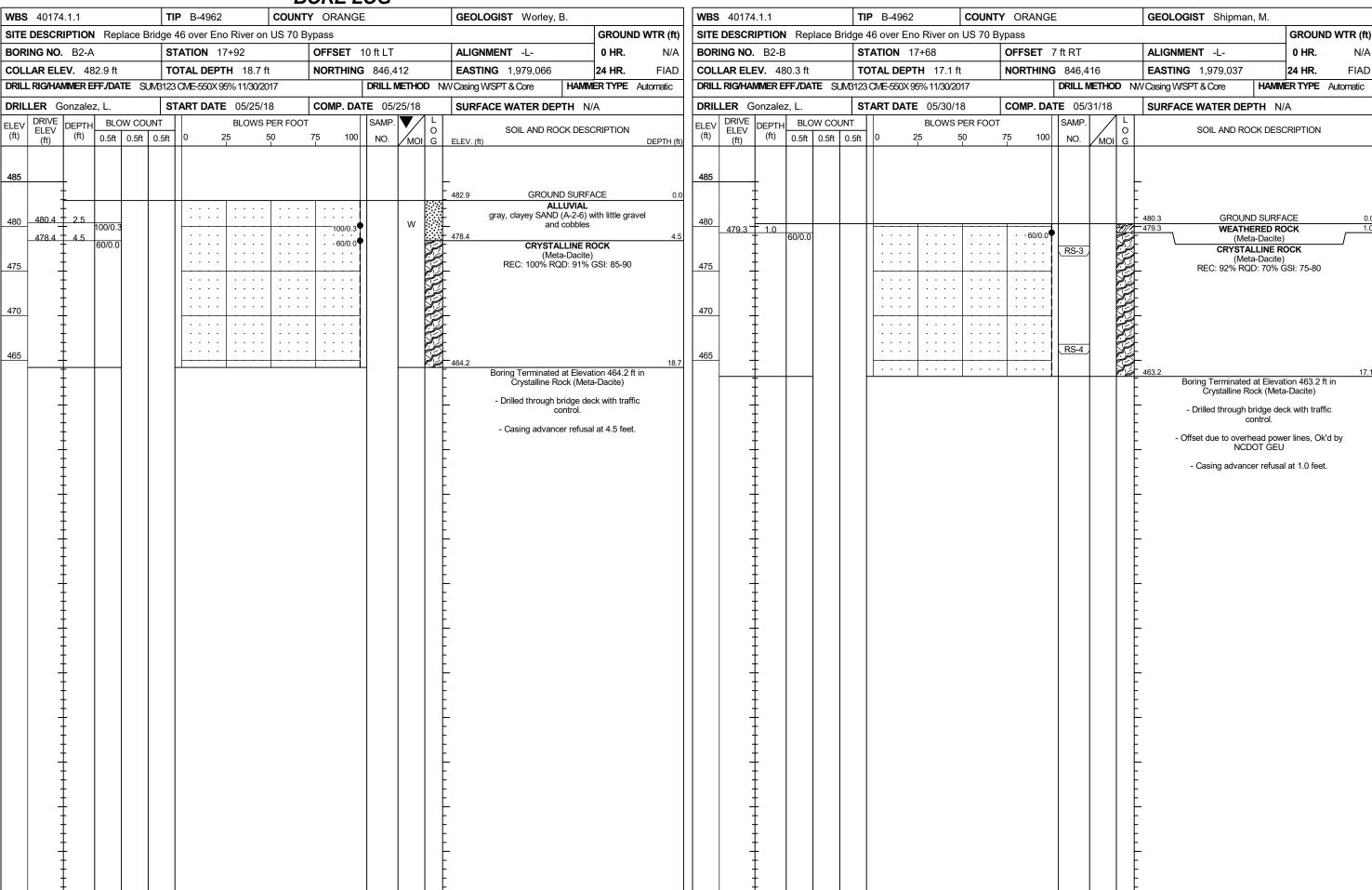
GROUND WTR (ft)

N/A

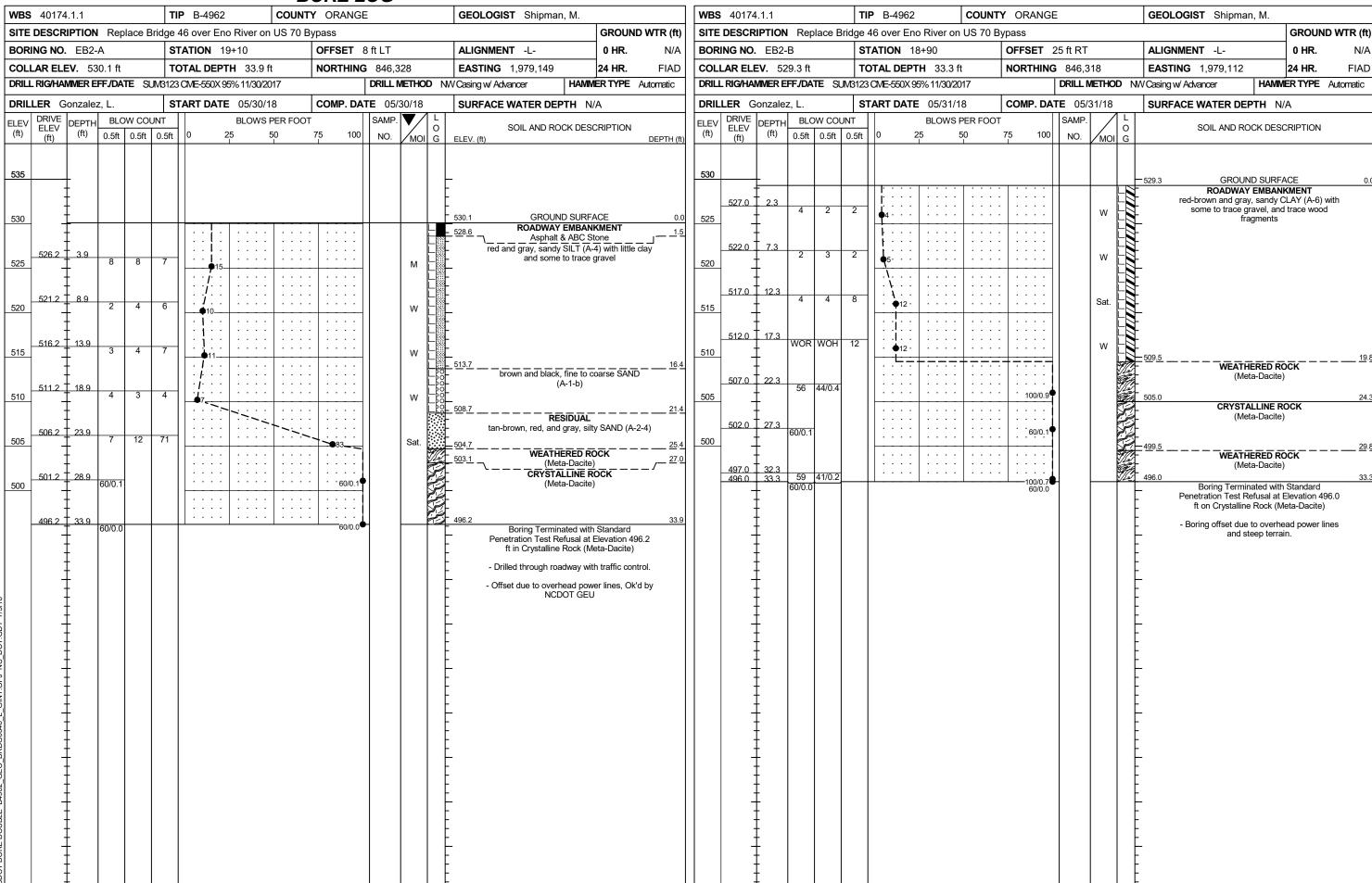
FIAD

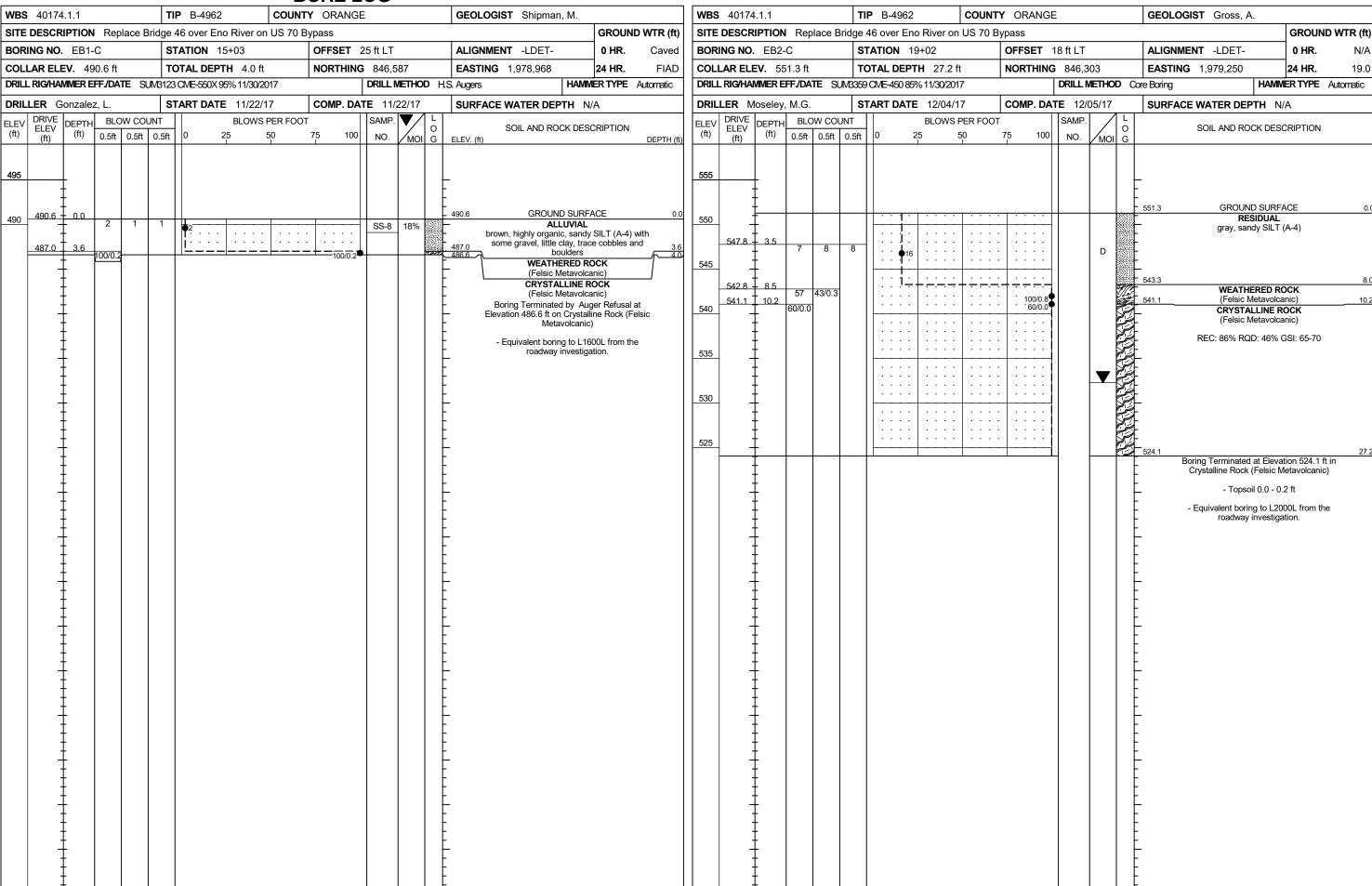
HAMMER TYPE Automatic

													<u>.OG</u>	1										, —																							
WB	401	74.1.1			TIP	B-496	62		COU	NTY	OR	ANGE	Ε			GE	OLOG	IST S	hipmar	n, M.				W	/BS	40174	4.1.1				TIP	B-496	62		COUN	TY	ORA	NGE				GEOL	OGIST	Shipma	an, M.		
SIT	DESC	RIPTIC	N Re	place Bri	dge 46	over l	Eno Riv	er on	US 70												GRO	UND V	VTR (ft	S	ITE C	DESCF	RIPTIC	N Re	eplace	e Brid	lge 46	over l	Eno Riv	er on l	JS 70											GROU	ND WTF
во	ING N) . B1-	-A		STA	TION	16+91				OFFS	ET 1	10 ft L	Т		ALI	GNME	ENT -L			0 HF	₹.	N/A	В	ORIN	IG NO	. B1-	-B			STA	TION	16+87			OF	FSE	T 12	2 ft RT			ALIGN	MENT	-L-		0 HR.	
		LEV. 4					PTH				NORT		846					1,978			24 HF		FIAD					481.1 f			<u> </u>		PTH 2			NC	DRTH		846,4					978,975		24 HR.	
DRII	L RIG/H	AMMER	EFF./D	ATE SUN	/B123 C	VIE-550.	X 95% 1	1/30/201	17				DRILL	. METHO	DD N	N Casir	ng W/SF	PT & Cor	re	HAMIN	/IER TYF	PE Au	tomatic	DI	RILL	RIG/HA	MMER	EFF./D	ATE	SUMB	3123 CN	VIE-550.	X 95% 11	/30/201	7				DRILL N	METHOD) NW	Casing \	WSPT&0	Core	HAMIN	ER TYPE	Automa
DRI	LER	Gonzal	lez, L.		STA	RT DA	TE 0	5/23/18	3	(COMF	P. DAT	TE 0	5/23/18	1	SUI	RFACE	E WATE	ER DEF	PTH N	/A			D	RILL	ER G	Sonzal	lez, L.			STA	RT DA	TE 05	/22/18		CC	OMP.	DAT	E 05/2	22/18		SURF	ACE WA	ATER DE	PTH 0.	5ft	
CO		NQ2			тот	AL RU	N 18.																	С		SIZE	NQ2						N 18.2														
ELE\((ft)	RUN ELEV (ft)	DEPT (ft)	H RUN (ft)	DRILL RATE (Min/ft)	REC. (ft)	RQD (ft) %	SAMI NO.	P. REG	C. RC	A QD ft)	L O G E	ELEV. (f	ft)		I	DESCF	RIPTION	N AND R	REMARK	S			DEPTH (f	EL (1	EV ft)	RUN ELEV (ft)	DEPT (ft)	H RUN	N RA	RILL ATE (lin/ft)	REC. (ft)	UN RQD (ft) %	SAMP NO.	REC (ft)	RATA RQI (ft) %	O G					DE	ESCRIP'	TION ANI	D REMARI	KS		
483.	483.6	1.4	3.5	0:45/0 5	(2.6)	(1.5)		(17	4) (14	16)		83.6				Beg	gin Co	ring @ ALLINE I	1.4 ft				1.		8.8	478.8	2.3	3 2	N=6	60/0 O	(2.8)	(2.6)		(17	5) (13.3	3)	4 479	ΩΩ				Begin	Coring	@ 2.3 ft NE ROCK			
480		4.9		3:13/1.0 2:50/1.0 6:00/1.0	74% (4.8) 96%	43%	RS-		.4) (14 % 79	4.0) N		603.0	gray spa	and whi	te, mod TA-DA	derate t	to fresh minor in	weather on-oxide angle. SI = 80-8	ring, hard staining	d, close t g, fractur	to wide f es most	fracture tly high				475.6 474.2	5.5	1.4	0:4 3:0 2:5 3:1 3:2	19/0.2 15/1.0 58/1.0 16/1.0 24/1.0	(2.8) 88% (1.3) 93%	(2.6) 81% (1.3) 93% (2.3)		96%	5) (13.3 6 73%			5.0	white a spacing	and gray g, META	r, modei A-DACIT	rate to fi	resh weat or iron-oxion 14.0 fe GSI = 75	thering, ha de staining et.	ird, close t g, heavily	o wide fra fractured	cture 12.2 -
475	475.1	9.9	5.0	3:21/1.0 4:33/1.0 6:42/1.0))) (5.0)	(4.6)	-																	4	70	470.6	10.5	- 1	2:4 2:3 2:3 2:1	86/0.6 16/1.0 30/1.0 11/1.0 /	94%	(2.3) 64% (1.4) 47%	_														
470	470.1	14.9	5.0	4:55/1.0 4:58/1.0 6:18/1.0 4:46/1.0 4:31/1.0			RS-2	2																_40	65	467.6	+	2.0	1:4 1:4 2:5 2:2	57/1.0 14/1.0 17/1.0 58/1.0 25/1.0	(2.0) 100% (4.4)	(0.7) 35% (4.4) 100%															
	465.1	19.9		4:31/1.0 7:06/1.0 5:42/1.0 5:38/1.0 6:35/1.0)	100%						65.1											19.	9		461.2 460.6	+ + + 19.9 + 20.5	0.6	2:3 2:3 2:3 0:4	21/1.0 34/1.0 25/1.0 30/1.0 10/0.4	100%	(0.6)					460	0.6	Roring	a Tormin	vated at	Elovatio	on 460 6 f	ft in Crysta	allino Pock	(Moto Dr	ucito)
		‡									-		Bori	•				65.1 ft in	•		,	Dacite)				_	Ī			13/0.0	100%	100%					E		БОППО	-				eck with tra		•	icite)
		‡									-						•	lge deck cer refus			Ol.						<u> </u>										-			-		•	•	efusal at 2.		u.	
		† † †									- - - - -															- -	‡ - - -										-										
		† †									-																 										- - -										
		‡ ‡									- - - -															- -	I I I										[- -										
		<u> </u>									- - -																‡ ‡										- - - -										
81/8/		‡ ‡									- - -																‡ + +										-										
DOI.601		‡ <u>‡</u>									- - - -																† † †										- - -										
NI.GPJ N.C.		†									-																 										- - -										
30046 L_G		Ţ									-															- - -	† - -										-										
GEO_BRU		†									-															- - -	† †										<u>-</u> - -										
SDOT CORE DOUBLE B4962_GEO_BRDG0046_L_GINT.GPJ NC_DOT.GDT 7/3/18		+									- - - -															- - -	 																				
1 P P P P P P P P P P P P P P P P P P P		‡									- - - -															- -	‡ ‡										<u>-</u> - -										
		+																								- -	<u> </u>										E										



	COL	RE LOG									
WBS 40174.1.1	TIP B-4962 COUNTY (ORANGE GI	EOLOGIST Worley, B.		WBS 40174.1.1		TIP B-4962	COUNT	Y ORANGE	GEOLOGIST Shipman, N	1.
SITE DESCRIPTION Replace Bridge	ge 46 over Eno River on US 70 Bypa	ass		ROUND WTR (ft)	SITE DESCRIPTION	Replace Brid	ge 46 over Eno River	on US 70 B	Bypass		GROUND WTR (ft)
BORING NO. B2-A	STATION 17+92 OF	FFSET 10 ft LT AL	LIGNMENT -L-	0 HR. N/A	BORING NO. B2-B		STATION 17+68		OFFSET 7 ft RT	ALIGNMENT -L-	0 HR. N/A
COLLAR ELEV. 482.9 ft	TOTAL DEPTH 18.7 ft NO	ORTHING 846,412	ASTING 1,979,066 2 4	4 HR. FIAD	COLLAR ELEV. 480).3 ft	TOTAL DEPTH 17.1	1 ft	NORTHING 846,416	EASTING 1,979,037	24 HR. FIAD
DRILL RIG/HAMMER EFF/DATE SUM3	123 CME-550X 95% 11/30/2017	DRILL METHOD NW Cas	sing W/SPT & Core HAMMER	RTYPE Automatic	DRILL RIG/HAMMER EF	F./DATE SUM	3123 CME-550X 95% 11/30	/2017	DRILL METHOD	NW Casing W/SPT & Core F	AMMER TYPE Automatic
DRILLER Gonzalez, L.	START DATE 05/25/18 CC	OMP. DATE 05/25/18 SU	JRFACE WATER DEPTH N/A		DRILLER Gonzalez	, L.	START DATE 05/30)/18	COMP. DATE 05/31/18	SURFACE WATER DEPTH	I N/A
CORE SIZE NQ2	TOTAL RUN 14.2 ft				CORE SIZE NQ2		TOTAL RUN 16.1 ft				
ELEV RUN ELEV (ft) DEPTH RUN (ft) PRILL RATE (Min/ft)	RUN	DESC ELEV. (ft)	CRIPTION AND REMARKS	DEPTH (ft)	ELEV RUN DEPTH (ft)	RUN (ft) DRILL RATE (Min/ft)	RUN REC. RQD SAMP. (ft) (ft) NO.	STRATA REC. RQD (ft) (ft) % %	L O G	DESCRIPTION AND REMARKS	
478.4		В	egin Coring @ 4.5 ft		479.3					Begin Coring @ 1.0 ft	
478.4	(4.2) (3.7) (14.2) (12.9) (15.0) (100% 100%	478.4 light to dark gray and whi	egin Coring @ 4.5 ft CRYSTALLINE ROCK ite, fresh, hard, moderately close to w spacing, META-DACITE. GSI = 85-90 evation 464.2 ft in Crystalline Rock (M bugh bridge deck with traffic control. Ing advancer refusal at 4.5 feet.	18.7		4.0 2:35/1.0 2:55/1.0 2:55/1.0 2:11/1.0 2:38/1.0 1:39/1.0 2:28/1.0 2:43/1.0	(4.0) (4.0) 100% 100%		close to modera Meta-And 463.2 Boring Terminate - Dri - Offset du	Begin Coring @ 1.0 ft CRYSTALLINE ROCK white, and black, moderate to fresh wately close fracture spacing, Meta-Dadesite 12.1 - 12.7 feet, minor iron-oxic GSI = 75-80 and at Elevation 463.2 ft in Crystalline at the content of the c	cite, small layer of de staining. 17.1 Rock (Meta-Dacite) control.
5		- - - -							-		





									C	<u>Oł</u>	E LOG			
WBS 40	174.1.1				TIP	B-496	52	C	OUNT	Y (ANGE	GEOLOGIST Gross, A.		
SITE DES	CRIPTIC	N R	epla	ce Brid	ge 46	over E	no River	on US	3 70 B	ypas			GROUN	D WTR (ft)
BORING N	NO. EB	2-C			STA	ΓΙΟΝ	19+02			OF	ET 18 ft LT	ALIGNMENT -LDET-	0 HR.	N/A
COLLAR							PTH 27.			NO	THING 846,303	EASTING 1,979,250	24 HR.	19.0
DRILL RIG/	/HAMMER	EFF./I	DATE	SUMB	359 CN	/IE-450 8	85% 11/30/	2017			DRILL METHOD Co	ore Boring HAN	MER TYPE	Automatic
DRILLER	Mosele	у, М.	G.		STAF	RT DA	TE 12/0	4/17		СО	P. DATE 12/05/17	SURFACE WATER DEPTH	N/A	
CORE SIZ							N 17.0 f	t						
ELEV RUI (ft) ELE (ft)	EV DEPT	H RU	ואון	DRILL RATE (Min/ft)	REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	RATA RQD (ft) %	L O G	[ELEV. (ft)	DESCRIPTION AND REMARKS		DEPTH (ft
541.07	1 + 10 2	,	0 14	/CO/O O	(0.0)	(0.0)		(44.7)	(7.0)	- C		Begin Coring @ 10.2 ft		10.0
	5.1 16.2 	5.0	0 1 3 4 0 1 3 3 4	(=60/0.0 6:59/1.0 2:01/1.0 2:37/1.0 2:37/1.0 3:15/1.0 3:42/1.0 1:54/1.0 3:38/1.0 4:50/1.0 3:31/1.0	(0.8) _80%_/ (3.9) 78% (4.1) 82%	(0.0) 0% / (1.3) 26% (1.9) 38%		(14.7) 86%	(7.8) 46%			CRYSTALLINE ROCK ery slightly weathered, hard, very clo- cture spacing, weakly foliated, Meta-I GSI = 65-70		10.2 ely
525 525	5.1 26.2	5.0	0 3 4 4 3 3	3:13/1.0 4:23/1.0 4:12/1.0 3:33/1.0 3:15/1.0	100%									
524	1.1 T 27.2 I	2 1.0	0 3	3:46/1.0	(0.9) \ 90% /	(0.6) \ 60% /					524.1 Boring Terminat	ed at Elevation 524.1 ft in Crystalline	Rock (Felsic	27.2
	1											Metavolcanic)		
	+											- Topsoil 0.0 - 0.2 ft		
	+++++++++++++++++++++++++++++++++++++++													

SHEET 17

ROCK CORE UNIAXIAL COMPRESSIVE STRENGTH TEST ASTM D-7012-10 METHOD C

Job No.: G17017.00 Job Name: Bridge No. 46 over Eno River

 Date:
 6/11/2018
 Sample No.:
 RS-1

 Boring No.:
 B1-A
 Depth (ft):
 3.7-4.7

 Description:
 TIP No. B-4962, WBS No. 40174.1.1

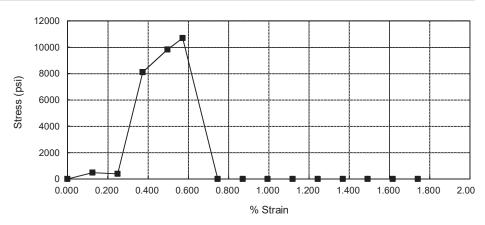
 Length (in.):
 4.020 Volume (in³):
 12.40289979

 Diameter (in.):
 1.982 Volume (cf):
 0.007177604

 Area (sq. in.):
 3.085 Unit Weight (pcf):
 167.9976

Compressive Strength (psi): 10693

			Compressive	Young's
Deflection (in.)	Strain (%)	Load (lbf)	Strength (psi)	Modulus (psi)
	,	, i	<u> </u>	, ,
0.000	0.000	0	0.0	
0.005	0.124	1490	482.9	388,280
0.010	0.249	1190	385.7	155,051
0.015	0.373	24980	8096.5	2,169,852
0.020	0.498	30310	9824.0	1,974,626
0.023	0.572	32990	10692.6	1,868,888
0.030	0.746		0.0	0
0.035	0.871		0.0	0
0.040	0.995		0.0	0
0.045	1.119		0.0	0
0.050	1.244		0.0	0
0.055	1.368		0.0	0
0.060	1.493		0.0	0
0.065	1.617		0.0	0
0.070	1.741		0.0	0



Notes:

Young's modulus is calculated using the secant modulus at each data interval per Figure 2 (C) in ASTM D 7012.

NCDOT Cert No. 105-02-0803

SHEET 18

ROCK CORE UNIAXIAL COMPRESSIVE STRENGTH TEST ASTM D-7012-10 METHOD C

Job No.: G17017.00 Job Name: Bridge No. 46 over Eno River

 Date:
 6/11/2018
 Sample No.:
 RS-2

 Boring No.:
 B1-A
 Depth (ft):
 11.3-12.2

 Description:
 TIP No. B-4962, WBS No. 40174.1.1

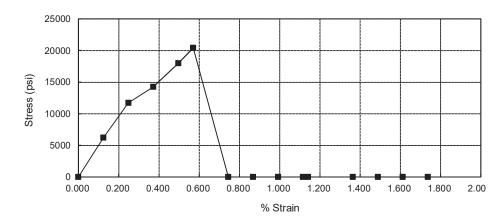
 Length (in.):
 4.030 Volume (in³):
 12.44630261

 Diameter (in.):
 1.983 Volume (cf):
 0.007202721

 Area (sq. in.):
 3.088 Unit Weight (pcf):
 169.5193

Compressive Strength (psi): 20392

			Compressive	Young's
Deflection (in.)	Strain (%)	Load (lbf)	Strength (psi)	Modulus (psi)
0.000	0.000	0	0.0	
0.005	0.124	19210	6220.0	5,013,339
0.010	0.248	36120	11695.3	4,713,217
0.015	0.372	43930	14224.1	3,821,551
0.020	0.496	55490	17967.2	3,620,383
0.023	0.571	62980	20392.4	3,573,095
0.030	0.744		0.0	0
0.035	0.868		0.0	0
0.040	0.993		0.0	0
0.045	1.117		0.0	0
0.046	1.141		0.0	0
0.055	1.365		0.0	0
0.060	1.489		0.0	0
0.065	1.613		0.0	0
0.070	1.737		0.0	0



Notes:

Young's modulus is calculated using the secant modulus at each data interval per Figure 2 (C) in ASTM D 7012.

NCDOT Cert No. 105-02-0803

ROCK CORE UNIAXIAL COMPRESSIVE STRENGTH TEST ASTM D-7012-10 METHOD C

Job No.: G17017.00 Job Name: Bridge No. 46 over Eno River

 Date:
 6/11/2018
 Sample No.:
 RS-3

 Boring No.:
 B2-B
 Depth (ft):
 2.5-3.1

 Description:
 TIP No. B-4962, WBS No. 40174.1.1

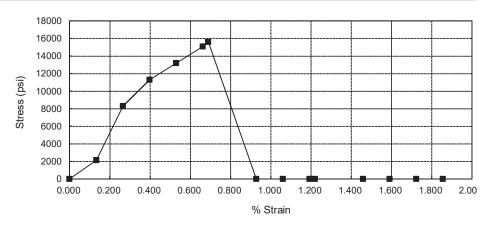
 Length (in.):
 3.771 Volume (in³):
 10.23540881

 Diameter (in.):
 1.859 Volume (cf):
 0.005923269

 Area (sq. in.):
 2.714 Unit Weight (pcf):
 168.5488

Compressive Strength (psi): 15603

			Compressive	Young's
Deflection (in.)	Strain (%)	Load (lbf)	Strength (psi)	Modulus (psi)
0.000	0.000	0	0.0	
0.005	0.133	5810	2140.6	1,614,411
0.010	0.265	22510	8293.3	3,127,400
0.015	0.398	30630	11284.9	2,837,028
0.020	0.530	35740	13167.6	2,482,747
0.025	0.663	40910	15072.3	2,273,512
0.026	0.689	42350	15602.9	2,263,018
0.035	0.928		0.0	0
0.040	1.061		0.0	0
0.045	1.193		0.0	0
0.046	1.220		0.0	0
0.055	1.458		0.0	0
0.060	1.591		0.0	0
0.065	1.724		0.0	0
0.070	1.856		0.0	0



Notes:

Young's modulus is calculated using the secant modulus at each data interval per Figure 2 (C) in ASTM D 7012.

NCDOT Cert No. 105-02-0803

ROCK CORE UNIAXIAL COMPRESSIVE STRENGTH TEST
ASTM D-7012-10 METHOD C

SHEET 19

Job No.: G17017.00 Job Name: Bridge No. 46 over Eno River

 Date:
 6/11/2018
 Sample No.:
 RS-4

 Boring No.:
 B2-B
 Depth (ft):
 13.5-14.2

 Description:
 TIP No. B-4962, WBS No. 40174.1.1

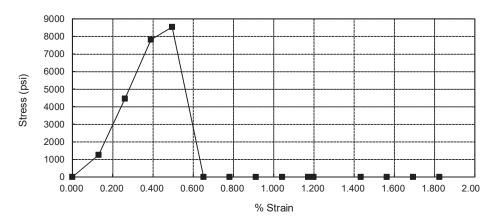
 Length (in.):
 3.838 Volume (in³):
 10.43968989

 Diameter (in.):
 1.861 Volume (cf):
 0.006041487

 Area (sq. in.):
 2.720 Unit Weight (pcf):
 164.2311

Compressive Strength (psi): 8525

			Compressive	Young's
Deflection (in.)	Strain (%)	Load (lbf)	Strength (psi)	Modulus (psi)
0.000	0.000	0	0.0	
0.005	0.130	3410	1253.6	962,292
0.010	0.261	12130	4459.4	1,711,525
0.015	0.391	21250	7812.3	1,998,895
0.019	0.495	23190	8525.5	1,722,144
0.025	0.651		0.0	0
0.030	0.782		0.0	0
0.035	0.912		0.0	0
0.040	1.042		0.0	0
0.045	1.172		0.0	0
0.046	1.199		0.0	0
0.055	1.433		0.0	0
0.060	1.563		0.0	0
0.065	1.694		0.0	0
0.070	1.824		0.0	0



Notes:

Young's modulus is calculated using the secant modulus at each data interval per Figure 2 (C) in ASTM D 7012.

NCDOT Cert No. 105-02-0803



CORE PHOTOGRAPHS

B1-A 1.4 - 19.9 FEET

RS-1: 3.7' - 4.5'

4.9

9.9





FEET

B1-B 2.3 - 19.3 FEET





FEET



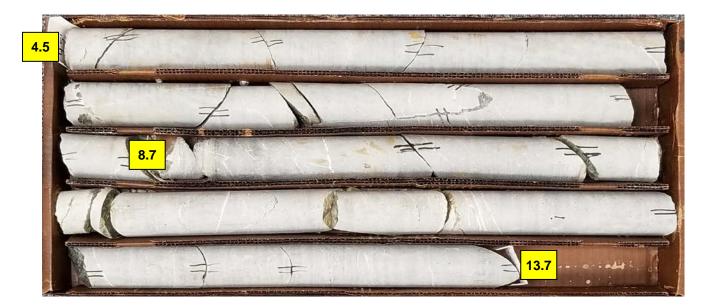
CORE PHOTOGRAPHS

B1-B (CONTINUED)

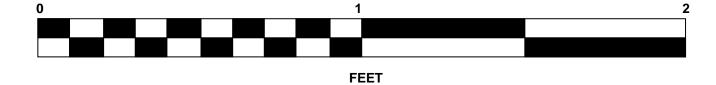
19.3 - 20.5 FEET



B2-A 4.5 - 18.7 FEET







FEET

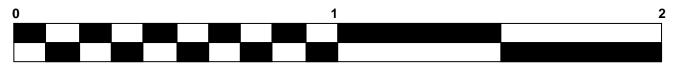
SUMMIT DESIGN AND ENGINEERING SERVICES

CORE PHOTOGRAPHS

B2-B 1.0 - 17.1 FEET

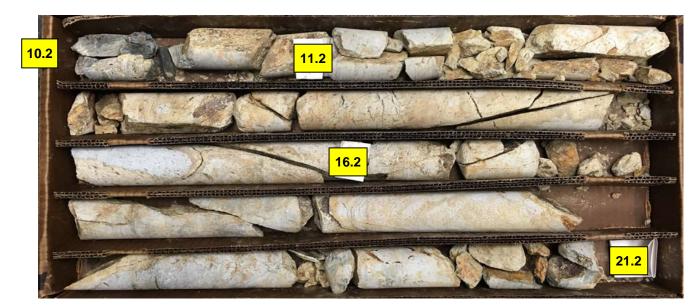
7.1 7.1

11.1 RS-4: 13.5' - 14.2'



FEET

EB2-C 10.2 - 27.2 FEET







FEET

SITE PHOTOGRAPHS

Bridge No. 46 over the Eno River on US 70 Bypass



Standing near End Bent 2 facing toward End Bent 1



Standing near End Bent 2 facing toward End Bent 1